

Worksheet – Mostly Solid / Solid Fencing – Wind Loading Only

Site Location:

Customer:

Site and Geometrical Variables

IBC _____ ASCE 7- _____ Risk Category _____ Frost Depth _____ ft

Basic Wind Speed, $V_w =$ _____ mph

Wind Pressure, $q_w =$ _____ psf

Exposure Category _____ Topographical Factor, $K_{zt} =$ _____

Site Elevation, $Z_e =$ _____ ft Elevation Factor, $K_e =$ _____

Height of fence, $h =$ _____ ft Gap at bottom of fence, $g =$ _____ ft (zero if no gap)

Height of fencing material, $s = h - g =$ _____ ft

Velocity Pressure Exposure Coefficient, $K_z =$ _____

Wind & Axial Loading

Fence Run # _____ Length of Fence, B = _____ ft Post spacing, L = _____ ft

Post Type: Line Posts () Post near end or corner ()

Solidity Ratio, ϵ = _____ Inverted Fence Opening Reduction Factor, R_{1w} = _____

Case C Reduction Factor, R_{2w} = _____ Return Corner Reduction Factor, R_{3w} = _____

Force Height Adjustment Factor, F_{hw} = _____ Wind Force Coefficient, C_{fw} = _____

Wind area tributary to the post, $A_w = \epsilon s L = \frac{\quad}{\epsilon} \times \frac{\quad}{s} \times \frac{\quad}{L} = \frac{\quad}{A_w} \text{ ft}^2$

Dead Load of fencing materials, D_m _____ psf

Lateral and Axial Forces for Wind Loading

Wind Force to the post, $f_w = q_w K_z K_{zt} K_e R_{1w} F_{hw} C_{fw} A_w$

$f_w = \frac{\quad}{q_w} \times \frac{\quad}{K_z} \times \frac{\quad}{K_{zt}} \times \frac{\quad}{K_e} \times \frac{\quad}{R_{1w}} \times \frac{\quad}{F_{hw}} \times \frac{\quad}{C_{fw}} \times \frac{\quad}{A_w} = \frac{\quad}{f_w} \text{ lbs}$

The Axial Force supported by the post, $p_w = D_w s L$

$p_w = \frac{\quad}{D_m} \times \frac{\quad}{s} \times \frac{\quad}{L} = \frac{\quad}{p_w} \text{ lbs}$

$5 \times p_w = \text{_____ lbs (used for stability check)}$

Post Selection

Using the desired post size and the Fence Height, h , look through the post charts for post types that have F_a values larger than the f_w value, and P_a values larger than the p_w value. If the post has an * next to the P_a value, only use it if the P_a value is $\geq 5 \times p_w$ due to stability requirements.

Put in the size, post type, weight per foot, D_p and fence height, h and calculate p_w' to include the weight of the post. Add any additional weight if needed.

f_w / F_a is the bending strength ratio for Wind.

p_w' / P_a is the axial strength ratio for Wind

If the sum of the bending strength and axial strength ratios for both Wind and Wind & Ice loading are ≤ 1.0 , the post is acceptable.

O.D.	Post Type	D _p (lb/f)	h (ft)	p _w ' = p _w + (D _p × h)	
				p _w ' =	
Wind					
f _w ' = _____ =		p _w ' = _____ =		$\frac{f_w'}{F_a} + \frac{p_w'}{P_a} =$	
F _a =		P _a =			

O.D.	Post Type	D _p (lb/f)	h (ft)	p _w ' = p _w + (D _p × h)	
				p _w ' =	
Wind					
f _w ' = _____ =		p _w ' = _____ =		$\frac{f_w'}{F_a} + \frac{p_w'}{P_a} =$	
F _a =		P _a =			

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				p _w ' =	
Wind					
f _w ' = _____ =		p _w ' = _____ =		$\frac{f_w'}{F_a} + \frac{p_w'}{P_a} =$	
F _a =		P _a =			

Footing Sizing (non-constrained footings)

Design Footing Depth, $D =$ _____ ft Footing Diameter, $b =$ _____ ft

Lateral Bearing Pressure per foot of depth, $S =$ _____ psf / ft per geotechnical analysis or table 1806.2

Maximum Wind Force, $P = f_w =$ _____ lbs Post Height, $h =$ _____ ft

Modifier for Isolated Posts, $M = 2.0$ per IBC §1086.3.4

Allowable Lateral Soil Bearing Pressure for non-constrained footings, $S_1 = \frac{1}{3} D S M$

$$S_1 = \frac{1}{3} \times \frac{\quad}{D} \times \frac{\quad}{S} \times \frac{2.0}{M} = \frac{\quad}{S_1} \text{ psf} \quad \text{per IBC §1807.3.2.1}$$

Soil Bearing Factor, $A = 2.34 P / (S_1 b)$ per IBC §1807.3.2.1

$$A = 2.34 \times \frac{\quad}{P} \div \left(\frac{\quad}{S_1} \times \frac{\quad}{b} \right) = \frac{\quad}{A}$$

Minimum Depth, $d = \frac{1}{2} A \left(1 + \sqrt{1 + \frac{4.36 \times \frac{1}{2} h}{A}} \right)$ per Eq. 18-1, **modified for fencing**

$$d = \frac{1}{2} \times \frac{\quad}{A} \times \left(1 + \sqrt{1 + \left(4.36 \times \frac{1}{2} \times \frac{\quad}{h} \div \frac{\quad}{A} \right)} \right) = \frac{\quad}{d} \text{ ft}$$

Area of the bottom of the footing, $A_f = \pi \left(\frac{1}{2} b \right)^2 = 3.14 \times \left(\frac{1}{2} \times \frac{\quad}{b} \right)^2 = \frac{\quad}{A_f} \text{ ft}^2$

Footing Volume, $V = A_f D = \frac{\quad}{A_f} \times \frac{\quad}{D} = \frac{\quad}{V} \text{ ft}^3$

Weight of footing, $D_f = 150 V = 150 \times \frac{\quad}{V} = \frac{\quad}{D_f} \text{ lbs}$ (Typical Concrete weight is 150 lbs / ft³)

Axial Dead Load, $D_{\max} = D_f + p_w'$

$$D_{\max} = \frac{\quad}{D_f} + \frac{\quad}{p_w'} = \frac{\quad}{D_{\max}} \text{ lbs}$$

Maximum Vertical Foundation Pressure, $S_y =$ _____ psf per geotechnical analysis or table 1806.2

Maximum Axial Pressure on the soil, $s_y = D_{\max} / A_f = \frac{\quad}{D_{\max}} \div \frac{\quad}{A_f} = \frac{\quad}{s_y} \text{ psf}$

Actual to Allowable Soil Strength Ratio, $s_y / S_y = \frac{\quad}{s_y} \div \frac{\quad}{S_y} = \frac{\quad}{\quad} \leq 1.0$ is OK

s_y / S_y must be less than 1.0. If not, start over with a larger footing diameter, b