

Worksheet – Open Fencing – Wind Loading

Site Location:

Customer:

Site and Geometrical Variables

IBC _____ ASCE 7- _____ Risk Category _____ Frost Depth _____ ft

Basic Wind Speed, $V_w =$ _____ mph

Wind Pressure, $q_w =$ _____ psf

Exposure Category _____ Topographical Factor, $K_{zt} =$ _____

Site Elevation, $Z_e =$ _____ ft Elevation Factor, $K_e =$ _____

Height of fence, $h =$ _____ ft Gap at bottom of fence, $g =$ _____ ft (zero if no gap)

Height of fencing material, $s = h - g =$ _____ ft

Velocity Pressure Exposure Coefficient, $K_z =$ _____

Wind & Axial Loading

Fence Run # _____ Length of Fence, B = _____ ft Post spacing, L = _____ ft

Solidity Ratio, $\epsilon =$ _____ Inverted Fence Opening Reduction Factor, $R_{1w} = 1.0$

Force Height Adjustment Factor, $F_{hw} = 1.0$ Wind Force Coefficient, $C_{fw} =$ _____

Expected Post Diameter or Width, $\emptyset_p =$ _____ in

Top Rail Diameter or Width, $\emptyset_r =$ _____ in Mid Rail Diameter or Width, $\emptyset_m =$ _____ in

(\emptyset_r & \emptyset_m – set to Zero if not applicable for rails)

Wind Area of Post, $A_p = \emptyset_p / 12 \times h = \frac{\quad}{\quad} \div 12 \times \frac{\quad}{\quad} = \frac{\quad}{\quad} \text{ ft}^2$
 ($A_p = 0$ for solid / mostly solid fencing)

Wind Area of Rails, $A_r = (\emptyset_r / 12) \times 2 \times L + (\emptyset_m / 12) \times L$ ($A_r = 0$ if no rails)

$A_r = \left(\frac{\quad}{\emptyset_r} \div 12 \times 2 \times \frac{\quad}{L} \right) + \left(\frac{\quad}{\emptyset_m} \div 12 \times \frac{\quad}{L} \right) = \frac{\quad}{A_r} \text{ ft}^2$

Wind area tributary to the post, $A_w = \epsilon s L + A_p + A_r = \frac{\quad}{\epsilon} \times \frac{\quad}{s} \times \frac{\quad}{L} + \frac{\quad}{A_p} + \frac{\quad}{A_r} = \frac{\quad}{A_w} \text{ ft}^2$

Dead Load of fencing materials, D_m _____ psf

Lateral and Axial Forces for Wind Loading

Minimum wind force to the post, $f_{min} = (0.6) 16.0 A_w = 9.6 \times \frac{\quad}{A_w} = \frac{\quad}{f_{min}} \text{ lbs}$

Calculated Wind Force to the post, $f_w = q_w K_z K_{zt} K_e R_{1w} F_{hw} C_{fw} A_w$

$f_w = \frac{\quad}{q_w} \times \frac{\quad}{K_z} \times \frac{\quad}{K_{zt}} \times \frac{\quad}{K_e} \times 1.0 \times 1.0 \times \frac{\quad}{R_{1w}} \times \frac{\quad}{F_{hw}} \times \frac{\quad}{C_{fw}} \times \frac{\quad}{A_w} = \frac{\quad}{f_w} \text{ lbs}$

Maximum wind force to the post, $f_w' =$ maximum value of either f_{min} or $f_w = \frac{\quad}{f_w'} \text{ lbs}$

Axial force supported by the post, $p_a = D_m s L$

$p_w = \frac{\quad}{D_m} \times \frac{\quad}{s} \times \frac{\quad}{L} = \frac{\quad}{p_w} \text{ lbs}$

$5 \times p_w =$ _____ lbs (used for stability check)

Post Selection

Using the size of the desired post and the Fence Height, h , look through the post charts for post types that have an F_a value larger than the f_w value, and a P_a value larger than the p_w value. If the post has an * next to the P_a value, only use it if the P_a value is $\geq 5 \times p_w$ due to stability requirements.

Put in the O.D or size, post type, weight per foot, D_p and fence height, h and calculate p_w' to include the weight of the post. Add any additional weight if needed.

f_w' / F_a is the bending strength ratio for Wind

p_w' / P_a is the axial strength ratio for Wind

If the sum of the bending strength and axial strength ratios are ≤ 1.0 , the post is acceptable.

O.D.	Post Type	D _p (lb/f)	h (ft)	p _w ' = p _w + (D _p × h)	
				p _w ' =	
Wind					
f _w ' = _____ =		p _w ' = _____ =		f _w ' / F _a + p _w ' / P _a =	
F _a =		P _a =			

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Wind					
f _w ' = _____ =		p _w ' = _____ =		f _w ' / F _a + p _w ' / P _a =	
F _a =		P _a =			

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f _w ' = _____ =		p _w ' = _____ =		f _w ' / F _a + p _w ' / P _a =	
F _a =		P _a =			

Footing Sizing (non-constrained footings)

Design Footing Depth, $D = \underline{\hspace{2cm}}$ ft Footing Diameter, $b = \underline{\hspace{2cm}}$ ft

Lateral Bearing Pressure per foot of depth, $S = \underline{\hspace{2cm}}$ psf / ft per geotechnical analysis or table 1806.2

Maximum Wind Force, $P = f_w' = \underline{\hspace{2cm}}$ lbs Post Height, $h = \underline{\hspace{2cm}}$ ft

Modifier for Isolated Posts, $M = 2.0$ per IBC §1086.3.4

Allowable Lateral Soil Bearing Pressure for non-constrained footings, $S_1 = \frac{1}{3} D S M$

$$S_1 = \frac{1}{3} \times \frac{\underline{\hspace{2cm}}}{D} \times \frac{\underline{\hspace{2cm}}}{S} \times \frac{2.0}{M} = \frac{\underline{\hspace{2cm}}}{S_1} \text{ psf} \quad \text{per IBC §1807.3.2.1}$$

Soil Bearing Factor, $A = 2.34 P / (S_1 b)$ per IBC §1807.3.2.1

$$A = 2.34 \times \frac{\underline{\hspace{2cm}}}{P} \div \left(\frac{\underline{\hspace{2cm}}}{S_1} \times \frac{\underline{\hspace{2cm}}}{b} \right) = \frac{\underline{\hspace{2cm}}}{A}$$

Minimum Depth, $d = \frac{1}{2} A \left(1 + \sqrt{1 + \frac{4.36 \times \frac{1}{2} h}{A}} \right)$ per Eq. 18-1, **modified for fencing**

$$d = \frac{1}{2} \times \frac{\underline{\hspace{2cm}}}{A} \times \left(1 + \sqrt{1 + \left(4.36 \times \frac{1}{2} \times \frac{\underline{\hspace{2cm}}}{h} \div \frac{\underline{\hspace{2cm}}}{A} \right)} \right) = \frac{\underline{\hspace{2cm}}}{d} \text{ ft}$$

Area of the bottom of the footing, $A_f = \pi \left(\frac{1}{2} b \right)^2 = 3.14 \times \left(\frac{1}{2} \times \frac{\underline{\hspace{2cm}}}{b} \right)^2 = \frac{\underline{\hspace{2cm}}}{A_f} \text{ ft}^2$

Footing Volume, $V = A_f D = \frac{\underline{\hspace{2cm}}}{A_f} \times \frac{\underline{\hspace{2cm}}}{D} = \frac{\underline{\hspace{2cm}}}{V} \text{ ft}^3$

Weight of footing, $D_f = 150 V = 150 \times \frac{\underline{\hspace{2cm}}}{V} = \frac{\underline{\hspace{2cm}}}{D_f} \text{ lbs}$ (Typical Concrete weight is 150 lbs / ft³)

Axial Dead Load, $D_{\max} = D_f + p_w'$

$$D_{\max} = \frac{\underline{\hspace{2cm}}}{D_f} + \frac{\underline{\hspace{2cm}}}{p_w'} = \frac{\underline{\hspace{2cm}}}{D_{\max}} \text{ lbs}$$

Maximum Vertical Foundation Pressure, $S_y = \underline{\hspace{2cm}}$ psf per geotechnical analysis or table 1806.2

Maximum Axial Pressure on the soil, $s_y = D_{\max} / A_f = \frac{\underline{\hspace{2cm}}}{D_{\max}} \div \frac{\underline{\hspace{2cm}}}{A_f} = \frac{\underline{\hspace{2cm}}}{s_y} \text{ psf}$

Actual to Allowable Soil Strength Ratio, $s_y / S_y = \frac{\underline{\hspace{2cm}}}{s_y} \div \frac{\underline{\hspace{2cm}}}{S_y} = \underline{\hspace{2cm}} \leq 1.0$ is OK

s_y / S_y must be less than 1.0. If not, start over with a larger footing diameter, b