

Site Location:

Customer:

Site and Geometrical Variables

IBC _____ ASCE 7- _____ Risk Category _____ Frost Depth _____ ft

Basic Wind Speed, $V_w =$ _____ mph

Wind Pressure, $q_w =$ _____ psf

Exposure Category _____ Topographical Factor, $K_{zt} =$ _____

Site Elevation, $Z_e =$ _____ ft Elevation Factor, $K_e =$ _____

Height of fence, $h =$ _____ ft Gap at bottom of fence, $g =$ _____ ft (zero if no gap)

Height of fencing material, $s = h - g =$ _____ ft

Velocity Pressure Exposure Coefficient, $K_z =$ _____

Wind & Axial Loading

Fence Run # _____ Length of Fence, B = _____ ft Post spacing, L = _____ ft

Solidity Ratio, $\epsilon =$ _____ Inverted Fence Opening Reduction Factor, $R_{1w} = 1.0$

Force Height Adjustment Factor, $F_{hw} = 1.0$ Wind Force Coefficient, $C_{fw} =$ _____

Expected Post Diameter or Width, $\emptyset_p =$ _____ in

Top Rail Diameter or Width, $\emptyset_r =$ _____ in Mid Rail Diameter or Width, $\emptyset_m =$ _____ in

(\emptyset_r & \emptyset_m – set to Zero if not applicable for rails)

Wind Area of Post, $A_p = \emptyset_p / 12 \times h = \frac{\text{_____}}{12} \times \frac{\text{_____}}{\text{_____}} = \text{_____ ft}^2$
 ($A_p = 0$ for solid / mostly solid fencing)

Wind Area of Rails, $A_r = (\emptyset_r / 12) \times 2 \times L + (\emptyset_m / 12) \times L$ ($A_r = 0$ if no rails)

$A_r = \left(\frac{\text{_____}}{\emptyset_r} \div 12 \times 2 \times \frac{\text{_____}}{L} \right) + \left(\frac{\text{_____}}{\emptyset_m} \div 12 \times \frac{\text{_____}}{L} \right) = \text{_____ ft}^2$

Wind area tributary to the post, $A_w = \epsilon s L + A_p + A_r = \frac{\text{_____}}{\epsilon} \times \frac{\text{_____}}{s} \times \frac{\text{_____}}{L} + \frac{\text{_____}}{A_p} + \frac{\text{_____}}{A_r} = \text{_____ ft}^2$

Dead Load of fencing materials, D_m _____ psf

Lateral and Axial Forces for Wind Loading

Minimum wind force to the post, $f_{min} = (0.6) 16.0 A_w = 9.6 \times \frac{\text{_____}}{A_w} = \text{_____ lbs}$

Calculated Wind Force to the post, $f_w = q_w K_z K_{zt} K_e R_{1w} F_{hw} C_{fw} A_w$

$f_w = \frac{\text{_____}}{q_w} \times \frac{\text{_____}}{K_z} \times \frac{\text{_____}}{K_{zt}} \times \frac{\text{_____}}{K_e} \times 1.0 \times 1.0 \times \frac{\text{_____}}{R_{1w}} \times \frac{\text{_____}}{F_{hw}} \times \frac{\text{_____}}{C_{fw}} \times \frac{\text{_____}}{A_w} = \text{_____ lbs}$

Maximum wind force to the post, $f_w' = \text{maximum value of either } f_{min} \text{ or } f_w = \text{_____ lbs}$

Axial force supported by the post, $p_a = D_w s L$

$P_w = \frac{\text{_____}}{D_w} \times \frac{\text{_____}}{s} \times \frac{\text{_____}}{L} = \text{_____ lbs}$

$5 \times p_w = \text{_____ lbs}$ (used for stability check)

Post Selection

Using the diameter of the desired post size, O.D., and the Fence Height, h, look through the post charts for post types that have an F_a value larger than the f_w' value, and a P_a value larger than the p_w value. If the post has an * next to the P_a value, only use it if the P_a value is $\geq 5 \times p_w$ due to stability requirements.

Put in the O.D, post type, weight per foot, D_p and fence height, h and calculate p_w' to include the weight of the post. Add any additional weight if needed.

f_w' / F_a is the bending strength ratio for Wind

p_w' / P_a is the axial strength ratio for Wind

If the sum of the bending strength and axial strength ratios are ≤ 1.0 , the post is acceptable.

O.D.	Post Type	D _p (lb/f)	h (ft)	p _w ' = p _w + (D _p × h)	
				p _w ' =	
Wind					
f _w ' = _____ =		p _w ' = _____ =		$\frac{f_w'}{F_a} + \frac{p_w'}{P_a} =$	
F _a =		P _a =			

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F _a =		P _a =			

Footing Sizing (non-constrained footings)

Design Footing Depth, $D = \underline{\hspace{2cm}}$ ft Footing Diameter, $b = \underline{\hspace{2cm}}$ ft

Lateral Bearing Pressure per foot of depth, $S = \underline{\hspace{2cm}}$ psf / ft per geotechnical analysis or table 1806.2

Maximum Wind Force, $P = f_w' = \underline{\hspace{2cm}}$ lbs Post Height, $h = \underline{\hspace{2cm}}$ ft

Modifier for Isolated Posts, $M = 2.0$ per IBC §1086.3.4

Allowable Lateral Soil Bearing Pressure for non-constrained footings, $S_1 = \frac{1}{3} D S M$

$S_1 = \frac{1}{3} \times \frac{\hspace{1cm}}{D} \times \frac{\hspace{1cm}}{S} \times 2.0 = \frac{\hspace{1cm}}{S_1}$ psf per IBC §1807.3.2.1

Soil Bearing Factor, $A = 2.34 P / (S_1 b)$ per IBC §1807.3.2.1

$A = 2.34 \times \frac{\hspace{1cm}}{P} \div \left(\frac{\hspace{1cm}}{S_1} \times \frac{\hspace{1cm}}{b} \right) = \frac{\hspace{1cm}}{A}$

Minimum Depth, $d = \frac{1}{2} A \left(1 + \sqrt{1 + \frac{4.36 \frac{1}{2} h}{A}} \right)$ per Eq. 18-1, **modified for fencing**

$d = \frac{1}{2} \times \frac{\hspace{1cm}}{A} \times \left(1 + \sqrt{1 + \left(4.36 \times \frac{1}{2} \times \frac{\hspace{1cm}}{h} \div \frac{\hspace{1cm}}{A} \right)} \right) = \frac{\hspace{1cm}}{d}$ ft

Area of the bottom of the footing, $A_f = \pi \left(\frac{1}{2} b \right)^2 = 3.14 \times \left(\frac{1}{2} \times \frac{\hspace{1cm}}{b} \right)^2 = \frac{\hspace{1cm}}{A_f}$ ft²

Footing Volume, $V = A_f D = \frac{\hspace{1cm}}{A_f} \times \frac{\hspace{1cm}}{D} = \frac{\hspace{1cm}}{V}$ ft³

Weight of footing, $D_f = 150 V = 150 \times \frac{\hspace{1cm}}{V} = \frac{\hspace{1cm}}{D_f}$ lbs (Typical Concrete weight is 150 lbs / ft³)

Axial Dead Load, $D_{max} = D_f + p_w'$

$D_{max} = \frac{\hspace{1cm}}{D_f} + \frac{\hspace{1cm}}{p_w'} = \frac{\hspace{1cm}}{D_{max}}$ lbs

Maximum Vertical Foundation Pressure, $S_y = \underline{\hspace{2cm}}$ psf per geotechnical analysis or table 1806.2

Maximum Axial Pressure on the soil, $s_y = D_{max} / A_f = \frac{\hspace{1cm}}{D_{max}} \div \frac{\hspace{1cm}}{A_f} = \frac{\hspace{1cm}}{s_y}$ psf

Actual to Allowable Soil Strength Ratio, $s_y / S_y = \frac{\hspace{1cm}}{s_y} \div \frac{\hspace{1cm}}{S_y} = \underline{\hspace{1cm}} \leq 1.0$ is OK

s_y / S_y must be less than 1.0. If not, start over with a larger footing diameter, b