

Worksheet – Open / Mostly Solid / Solid Fencing – Wind and Wind & Ice Loading

Site Location:

Customer:

Site and Geometrical Variables

IBC _____ ASCE 7- _____ Risk Category _____ Frost Depth _____ ft

Basic Wind Speed, $V_w =$ _____ mph Nominal Ice Thickness, $t =$ _____ in

Gust / Concurrent Wind Speed for Ice Loading, $V_i =$ _____ mph

Wind Pressure, $q_w =$ _____ psf Wind on Ice Pressure, $q_i =$ _____ psf

Exposure Category _____ Topographical Factor, $K_{zt} =$ _____

Site Elevation, $Z_e =$ _____ ft Elevation Factor, $K_e =$ _____

Height of fence, $h =$ _____ ft Gap at bottom of fence, $g =$ _____ ft (zero if no gap)

Height of fencing material, $s = h - g =$ _____ ft

Velocity Pressure Exposure Coefficient, $K_z =$ _____

Wind & Axial Loading

Fence Run # _____ Length of Fence, B = _____ ft Post spacing, L = _____ ft

Post Type: Line Posts () Post near end or corner ()

Solidity Ratio, ϵ = _____ Inverted Fence Opening Reduction Factor, R_{1w} = _____

Case C Reduction Factor, R_{2w} = _____ Return Corner Reduction Factor, R_{3w} = _____

Force Height Adjustment Factor, F_{hw} = _____ Wind Force Coefficient, C_{fw} = _____

Expected Post Diameter or Width, \emptyset_p = _____ in (Set to Zero for solid / mostly solid fencing)

Top Rail Diameter or Width, \emptyset_r = _____ in Mid Rail Diameter or Width, \emptyset_m = _____ in

(\emptyset_r & \emptyset_m – set to Zero if not applicable for rails or for solid / mostly solid fencing)

Wind Area of Post, $A_p = \emptyset_p / 12 \times h = \frac{\text{_____}}{12} \times \frac{\text{_____}}{\text{_____}} = \text{_____} \text{ ft}^2$
 ($A_p = 0$ for solid / mostly solid fencing)

Wind Area of Rails, $A_r = (\emptyset_r / 12) \times 2 \times L + (\emptyset_m / 12) \times L$ ($A_r = 0$ for solid / mostly solid fencing)

$A_r = \left(\frac{\text{_____}}{\emptyset_r} \div 12 \times 2 \times \frac{\text{_____}}{L} \right) + \left(\frac{\text{_____}}{\emptyset_m} \div 12 \times \frac{\text{_____}}{L} \right) = \text{_____} \text{ ft}^2$

Wind area tributary to the post, $A_w = \epsilon s L + A_p + A_r = \frac{\text{_____}}{\epsilon} \times \frac{\text{_____}}{s} \times \frac{\text{_____}}{L} + \frac{\text{_____}}{A_p} + \frac{\text{_____}}{A_r} = \text{_____} \text{ ft}^2$

Dead Load of fencing materials, D_m _____ psf

Lateral and Axial Forces for Wind Loading

Wind Force to the post, $f_w = q_w K_z K_{zt} K_e R_{1w} F_{hw} C_{fw} A_w$

$f_w = \frac{\text{_____}}{q_w} \times \frac{\text{_____}}{K_z} \times \frac{\text{_____}}{K_{zt}} \times \frac{\text{_____}}{K_e} \times \frac{\text{_____}}{R_{1w}} \times \frac{\text{_____}}{F_{hw}} \times \frac{\text{_____}}{C_{fw}} \times \frac{\text{_____}}{A_w} = \text{_____} \text{ lbs}$

The Axial Force supported by the post, $p_a = D_w s L$

$P_w = \frac{\text{_____}}{D_w} \times \frac{\text{_____}}{s} \times \frac{\text{_____}}{L} = \text{_____} \text{ lbs}$

$5 \times p_w = \text{_____} \text{ lbs}$ (used for stability check)

Wind on Ice Loading

Fence Run # _____ Length of Fence, B = _____ ft Post spacing, L = _____ ft

Post Type: Line Posts() Post near end or corner()

Wind on Ice Force Variables

Iced Solidity Ratio, ϵ' = _____ Inverted Fence Opening Reduction Factor, R_{1i} = _____

Case C Reduction Factor, R_{2i} = _____ Return Corner Reduction Factor, R_{3i} = _____

Force Height Adjustment Factor, F_{hi} = _____ Wind Force Coefficient, C_{fi} = _____

Dead Load of Ice, D_i = _____ psf

Wind area tributary to the post in the iced condition, $A_i = \epsilon' s L$

$$A_i = \frac{\epsilon'}{\epsilon'} \times s \times L = \frac{\epsilon'}{\epsilon'} \text{ ft}^2$$

Wind on Ice Force to the post, $f_i = q_i K_z K_{zt} K_e R_{1i} F_{hi} C_{fi} A_i$

$$F_i = \frac{q_i}{q_i} \times \frac{K_z}{K_z} \times \frac{K_{zt}}{K_{zt}} \times \frac{K_e}{K_e} \times \frac{R_{1i}}{R_{1i}} \times \frac{F_{hi}}{F_{hi}} \times \frac{C_{fi}}{C_{fi}} \times \frac{A_i}{A_i} = \frac{F_i}{F_i} \text{ lbs}$$

Axial Force supported by the post, $p_i = (D_w + D_i) s L$

$$p_i = \left(\frac{D_w}{D_w} + \frac{D_i}{D_i} \right) \times s \times L = \frac{p_i}{p_i} \text{ lbs}$$

$5 \times p_i = \text{_____ lbs}$ (used for stability check)

Post Selection

Using the diameter of the desired post size, O.D., and the Fence Height, h, look through the post charts for post types that have F_a values larger than the f_w & f_i values, and P_a values larger than the p_w & p_i values. If the post has an * next to the P_a value, only use it if the P_a value is $\geq 5 \times p_i$ due to stability requirements.

Put in the O.D, post type, weight per foot, D_p and fence height, h and calculate p_w' and p_i' to include the weight of the post. Add any additional weight if needed.

f_w / F_a is the bending strength ratio for Wind. f_i / F_a is the bending strength ratio for Wind & Ice.
 p_w' / P_a is the axial strength ratio for Wind. p_i' / P_a is the axial strength ratio for Wind & Ice.

If the sum of the bending strength and axial strength ratios for both Wind and Wind & Ice loading are ≤ 1.0 , the post is acceptable.

O.D.	Post Type	D _p (lb/f)	h (ft)	p _w ' = p _w + (D _p × h) p _w ' =	p _i ' = p _i + (D _p × h) p _i ' =
Wind					
$f_w = \frac{\quad}{F_a} =$		$p_w' = \frac{\quad}{P_a} =$		$\frac{f_w}{F_a} + \frac{p_w'}{P_a} =$	
Wind & Ice					
$f_i = \frac{\quad}{F_a} =$		$p_i' = \frac{\quad}{P_a} =$		$\frac{f_i}{F_a} + \frac{p_i'}{P_a} =$	

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Wind					
$f_w = \frac{\quad}{F_a} =$		$p_w' = \frac{\quad}{P_a} =$		$\frac{f_w}{F_a} + \frac{p_w'}{P_a} =$	
Wind & Ice					
$f_i = \frac{\quad}{F_a} =$		$p_i' = \frac{\quad}{P_a} =$		$\frac{f_i}{F_a} + \frac{p_i'}{P_a} =$	

Footing Sizing (non-constrained footings)

Design Footing Depth, $D = \underline{\hspace{2cm}}$ ft Footing Diameter, $b = \underline{\hspace{2cm}}$ ft

Lateral Bearing Pressure per foot of depth, $S = \underline{\hspace{2cm}}$ psf / ft per geotechnical analysis or table 1806.2

Maximum Wind Force, $P = \text{maximum value of } f_w \text{ or } f_i = \underline{\hspace{2cm}}$ lbs Post Height, $h = \underline{\hspace{2cm}}$ ft

Modifier for Isolated Posts, $M = 2.0$ per IBC §1086.3.4

Allowable Lateral Soil Bearing Pressure for non-constrained footings, $S_1 = \frac{1}{3} D S M$

$$S_1 = \frac{1}{3} \times \frac{\hspace{1cm}}{D} \times \frac{\hspace{1cm}}{S} \times \frac{2.0}{M} = \frac{\hspace{1cm}}{S_1} \text{ psf} \quad \text{per IBC §1807.3.2.1}$$

Soil Bearing Factor, $A = 2.34 P / (S_1 b)$ per IBC §1807.3.2.1

$$A = 2.34 \times \frac{\hspace{1cm}}{P} \div \left(\frac{\hspace{1cm}}{S_1} \times \frac{\hspace{1cm}}{b} \right) = \frac{\hspace{1cm}}{A}$$

Minimum Depth, $d = \frac{1}{2} A \left(1 + \sqrt{1 + \frac{4.36 \frac{1}{2} h}{A}} \right)$ per Eq. 18-1, **modified for fencing**

$$d = \frac{1}{2} \times \frac{\hspace{1cm}}{A} \times \left(1 + \sqrt{1 + \left(4.36 \times \frac{1}{2} \times \frac{\hspace{1cm}}{h} \div \frac{\hspace{1cm}}{A} \right)} \right) = \frac{\hspace{1cm}}{d} \text{ ft}$$

Area of the bottom of the footing, $A_f = \pi \left(\frac{1}{2} b \right)^2 = 3.14 \times \left(\frac{1}{2} \times \frac{\hspace{1cm}}{b} \right)^2 = \frac{\hspace{1cm}}{A_f} \text{ ft}^2$

Footing Volume, $V = A_f D = \frac{\hspace{1cm}}{A_f} \times \frac{\hspace{1cm}}{D} = \frac{\hspace{1cm}}{V} \text{ ft}^3$

Weight of footing, $D_f = 150 V = 150 \times \frac{\hspace{1cm}}{V} = \frac{\hspace{1cm}}{D_f} \text{ lbs}$ (Typical Concrete weight is 150 lbs / ft³)

Axial Dead Load, $D_{\max} = D_f + p_i'$ for fencing plus ice loading. $D_{\max} = D_f + p_w'$ if there is no ice loading

$$D_{\max} = \frac{\hspace{1cm}}{D_f} + \frac{\hspace{1cm}}{p_i' \text{ or } p_w'} = \frac{\hspace{1cm}}{D_{\max}} \text{ lbs}$$

Maximum Vertical Foundation Pressure, $S_y = \underline{\hspace{2cm}}$ psf per geotechnical analysis or table 1806.2

Maximum Axial Pressure on the soil, $s_y = D_{\max} / A_f = \frac{\hspace{1cm}}{D_{\max}} \div \frac{\hspace{1cm}}{A_f} = \frac{\hspace{1cm}}{s_y} \text{ psf}$

Actual to Allowable Soil Strength Ratio, $s_y / S_y = \frac{\hspace{1cm}}{s_y} \div \frac{\hspace{1cm}}{S_y} = \underline{\hspace{1cm}} \leq 1.0$ is OK

s_y / S_y must be less than 1.0. If not, start over with a larger footing diameter, b